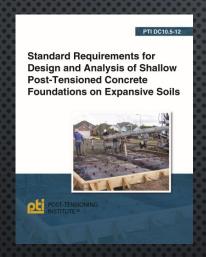
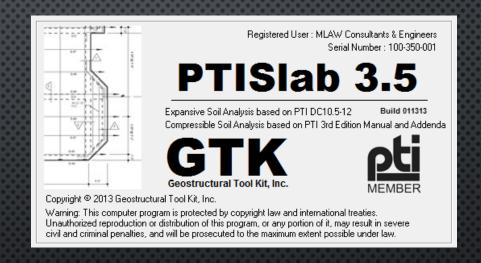
SIGNIFICANT CHANGES IN PTI DC10.5-12 AND PTISLAB 3.5





FOUNDATION PERFORMANCE ASSOCIATION – OCTOBER PRESENTATION
OCTOBER 9, 2013 – HOUSTON, TEXAS

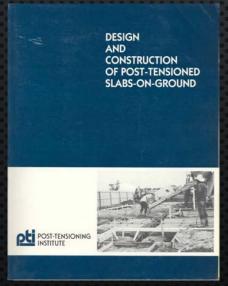
DEAN R. READ, P.E.

MLAW CONSULTANTS & ENGINEERS

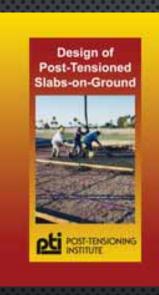
GEOSTRUCTURAL TOOL KIT, INC.



"1st Edition Manual" Published in 1980

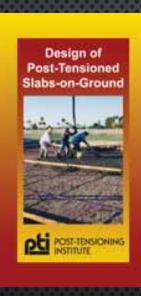


"2nd Edition Manual" Published in 1996



"3rd Edition Manual" and associated Standards
Published in December 2004

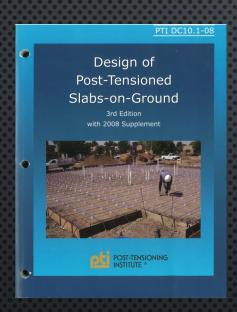
- Standard Requirements for Analysis of Shallow Concrete Foundations on Expansive Soils
- Standard Requirements for Design of Shallow Post-Tensioned Concrete Foundations on Expansive Soils



"3rd Edition Manual" and associated Standards subsequently modified

Addendum #1 Published in May 2007

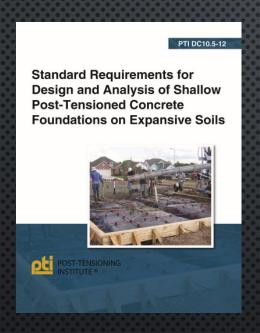
Addendum #2 Published in May 2008



"3rd Edition Manual with 2008 Supplement"

Includes both standards, both addenda and errata

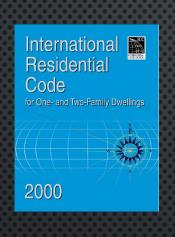
(three errata issued after publication of supplement)



Standard Requirements for Design and Analysis of Shallow Post-Tensioned Concrete Foundations on Expansive Soils

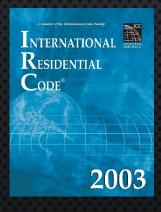
"Combined Standard"

This is not a design manual. It is written in mandatory code language with commentary. "User Guide" being developed.





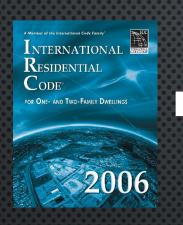




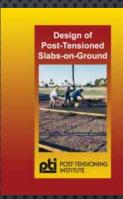












Standards



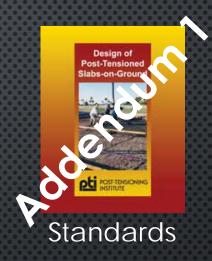
















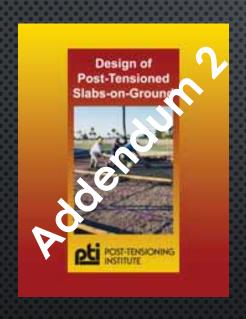


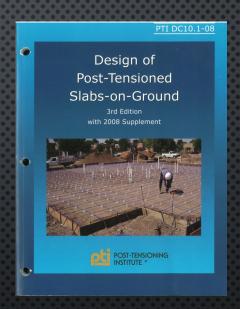
S IBC Image not available



Submitted for approval.







PTI documents not referenced in IRC or IBC

PTI DC10.5-12

Standard Requirements for Design and Analysis of Shallow Post-Tensioned Concrete Foundations on Expansive Soils



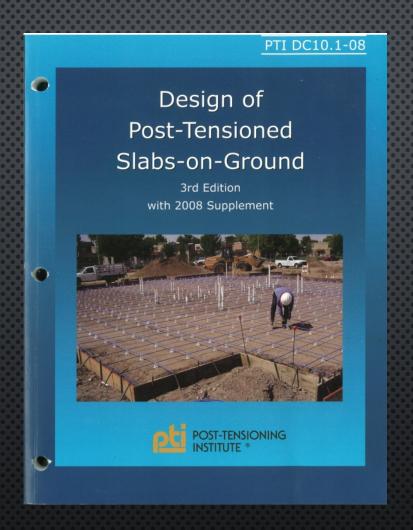


"Combined Standard"

Considered the "state of the art"



What do I use for compressible soils?



Should be used for compressible soils

APPLIED SHEAR FOR "SQUARE" DESIGN RECTANGLE





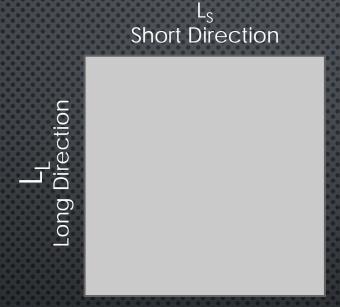
In 3rd Edition (and earlier), Center Lift V_S ≠ V_L for "square" design rectangle (Edge Lift V_S already equal to V_L for "square" design rectangle)



In Combined Standard, $V_S = V_L$ for "square" design rectangle

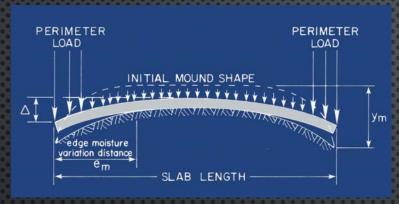
(this is consistent with the applied moment provisions)

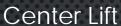
"SQUARE" DESIGN RECTANGLE

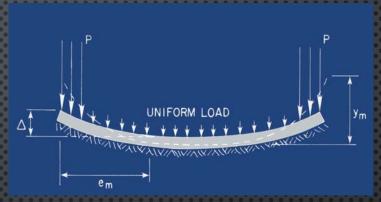


If L_L / L_S < 1.1 the design rectangle considered a "square"

APPLIED SHEAR FOR SOILS WITH Y_m <=1







Edge Lift



In 3^{rd} Edition, V_s and V_L are calculated using entire e_m length.

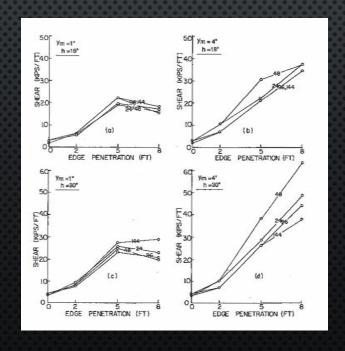


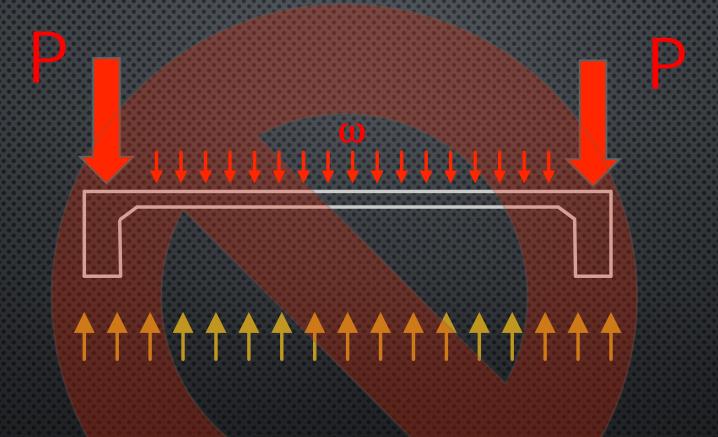
In Combined Standard, V_s and V_L are calculated by limiting e_m to 5 feet.

APPLIED SHEAR FOR SOILS WITH Y_m <=1

Combined Standard limit "caps" V_s and V_L for soils with low shrink / swell potential

Justification for limit comes from original research by Lytton and Wray:





The 3rd Edition (and earlier) method assumes foundation stiff enough to apply uniform pressure to soil.

6.2.4 — Soil-bearing pressure

Applied soil-bearing pressure shall be evaluated using generally accepted techniques and shall not exceed $q_{\rm allow}$ as specified by the LDP with geotechnical experience.

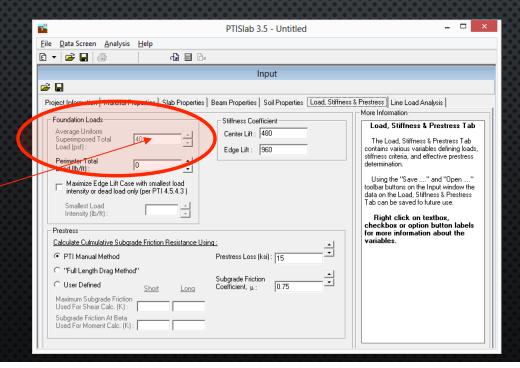
R6.2.4 — Soil-bearing pressure

Refer to PTI DC10.1-08² for one method of determining the applied soil-bearing pressure. Other generally accepted techniques may be used.

The "average uniform superimposed total load" (ω) variable is only used in the bearing calculations.

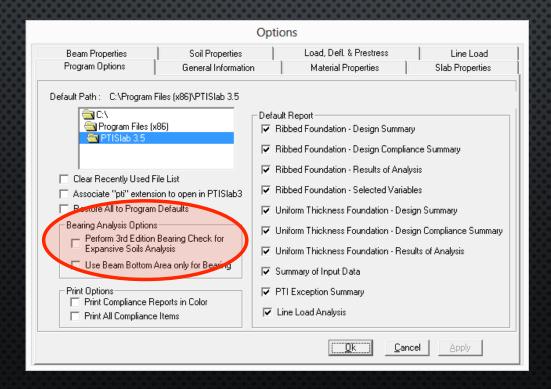
Since PTI no longer requires the "unrealistic" bearing analysis, this variable is no longer needed in PTISIab and has been disabled.

Most common question asked about PTISIab 3.5: "Why can't I enter a value here?"



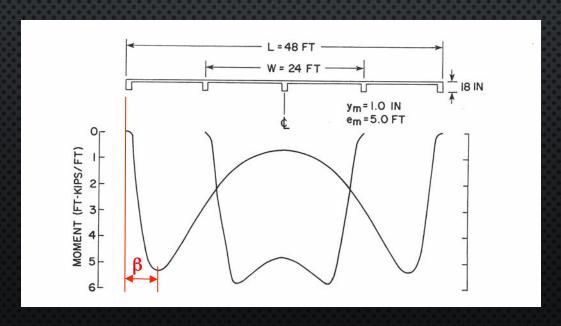
Why wasn't variable completely removed from PTISlab?

- It is still required for compressible soil analysis
- It can be "re-enabled" if an engineer wants to still perform the "unrealistic" analysis.

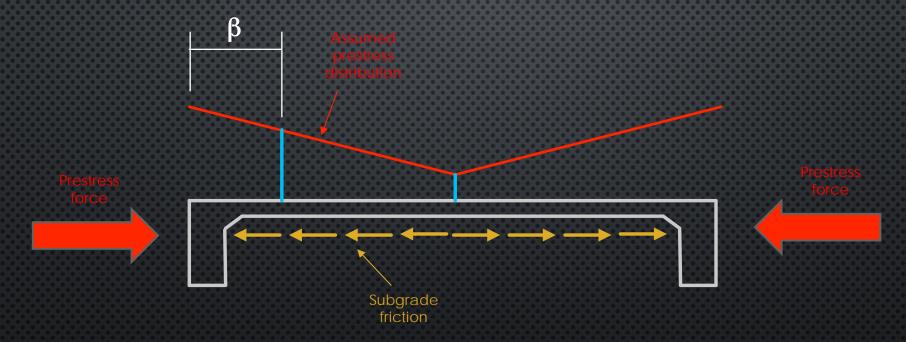


Based on the original analysis by Lytton and Wray, the location of the maximum moment "can closely be estimated by β . Maximum shear occurs within β of edge of slab.

β is a function of the relative stiffness of the soil and stiffened slab.



In PTI 2^{nd} and 3^{rd} Edition Manuals, P_r is calculated at mid-slab. $P_{r \text{ mid slab}} < P_{r \text{ beta}}$



In "Combined Standard" P_r is calculated at β

6.1.1 — Loss of prestress

Effective prestress force in the concrete after all losses shall be

$$P_{\cdot} = P_{\cdot} - ES - CR - SH - RE - SG$$

For determination of the minimum effective prestress force P, SG shall be calculated as follows:

$$SG = (W_{slab}/2000)\mu$$

For determination of the effective prestress force P_r used in the flexural and shear stress calculations, SG shall be calculated as follows

$$SG = (W_{slab}/2000)(\beta/L)(\mu)$$

where β and L are in the direction being considered.

The prestress at b is used in moment and shear stress calculations.

The prestress at mid-slab is still used for minimum effective prestress.

Concrete flexural stresses shall be calculated as follows

$$f = \frac{1000P_r}{A} \pm \frac{12,000M_{L,S}}{S_{T,B}} \pm \frac{1000P_re_p}{S_{T,B}}$$

In general, change decreases the applied tensile stresses but.....

if eccentricity of prestress is large, change can actually increase applied tensile stresses.

VERTICAL MOISTURE BARRIER



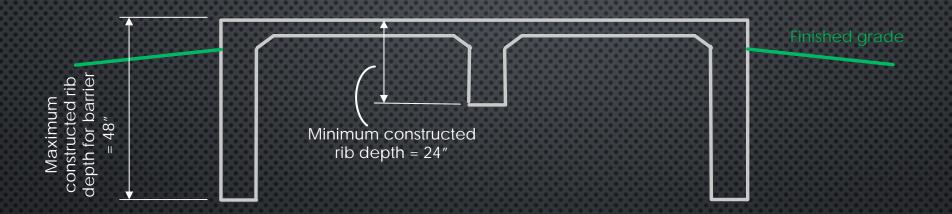


In 3rd Edition, the minimum barrier depth is 30 inches



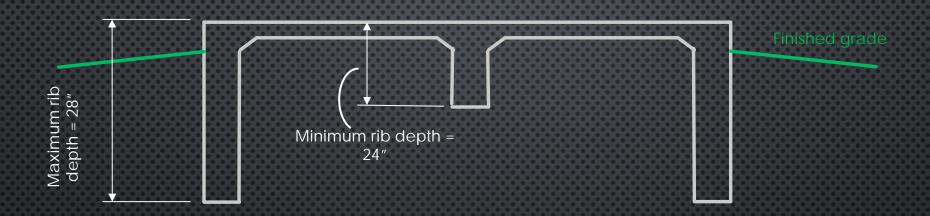
In "Combined Standard", the minimum barrier depth is 24 inches

VARIABLE RIB DEPTHS



What rib depth values do you use in PTISlab?

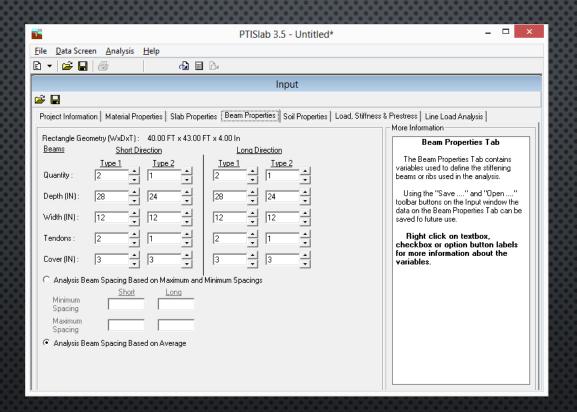
VARIABLE RIB DEPTHS



In analysis, ratio of deepest to shallowest rib depth cannot exceed 1.2 (not new in combined standard but misunderstood).

The difference in the constructed ribs are not limited to the 1.2 ratio

VARIABLE RIB DEPTHS



Typically Type 1 ribs used for perimeter ribs

Typically Type 2 used for interior ribs

OTHER CHANGES

- Explicitly permits use of horizontal barriers to reduce e_m and y_m. (5.3.2)
- Limits placed on effectiveness of vertical and horizontal barriers at reducing e_m and y_m. (5.3.1 and 5.3.2)
- Replaced specific PPM Chloride limit with limit determined by local experience and practice. (4.3.2.2)